GENERAL NOTES

General Notes

1. ALL WORK SHALL CONFORM TO THE CBC 2001.
2. THE FOLLOWING NOTES AND ALL OTHER TYPICAL DETAILS APPLY TO ALL DRAWINGS UNLESS NOTED OTHERWISE.
3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHENICAL AND ELECTRICAL DRAWINGS.
4. FRAMING CONDITIONS NOT SPECIFICALLY SHOWN SHALL BE FRAMED SIMILAR TO THE DETAILS SHOWN FOR THE RESPECTIVE MATERIALS.
5. PROVIDE OPENINGS AND SUPPORTS FOR MECHANICALS EQUIPMENT, DUCTS, PIPING, VENTS AND ETC., AS REQUIRED. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL OPENINGS AND EQUIPMENT WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS. ALL SUSPENDED EQUIPMENT TO BE PROVIDED WITH APPROVED AL BRACING.
6. CONTRACTOR MUST CLARIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE COMMENCING WORK. ARCHITECTURE SHALL BE INFORMED AND NOTIFIED ANY TYPE OF DISCREPANCIES.
7. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECENDENCE OVER THESE GENERAL NOTES.
8. DESIGN MATERIALS, EQUIPMENTS AND PRODUCTS OTHER THAN THOSE DESCRIBED BELOW OR INDICATED ON THE DRAWINGS MAY BE CONSIDERED FOR USE. PROVIDED PRIOR APPROVAL IS OBTAINED FROM THE OWNER, ARCHITECT/ENGINEER, AND THE APPLICABLE GOVERNING CODE AUTHORITY.
9. A. TYPE OF CONSTRUCTION : TYPE V , 2 WOOD STORY, 1 STORY CONC DECK.
   B. ROOF DEAD LOAD : 23 psf 3RD & 2ND FLOOR DEAD LOAD : 19 psf
   LIVE LOAD : 16 psf LIVE LOAD : 40 psf
   1ST FLOOR DEAD LOAD : 143 psf
   LIVE LOAD : 40 psf
   C. Z = 0.4 , I = 1.0 , Ca = 0.512 , Na = 1.3 , Nv = 1.6.
   D. BASIC WIND SPEED : 70 mph.
   WIND PRESSURE COEFFICIENT : 1.3

Foundations

1. ALLOWABLE FOUNDATION PRESSURE USED IN THE DEVELOPMENT AND DESIGN OF THIS STRUCTURE IS 2,000 PSF. FOR ALL FOOTING.
2. SOIL REPORT WAS PREPARED BY C.Y. GEOTECH, INC.
   (FILE NO.) CYG-06-4508 (DATE) MAY 10, 2006
3. ALL EARTH WORK SHALL BE DONE ACCORDING TO SOILS REPORT ATTATCHED.
4. FOUNDATIONS SHALL BE THE SIZE AND TYPE AS INDICATED ON THE DRAWINGS.
5. ALL EXCAVATIONS ARE TO BE INSPECTED AND APPROVED BY A SOILS ENGINEER PRIOR TO THE PLACEMENT OF ANY FILL OR REINFORCING STEEL.
6. SUB SOIL BELOW SLAB AND FOOTINGS SHALL BE PRESOAKED FOR 48 HOURS PRIOR TO PLACING CONCRETE.
7. CONCRETE CONTRACTOR TO INSURE A TRUE AND LEVEL FOUNDATION PRIOR TO PLACING CONCRETE.
8. CONCRETE CONTRACTOR TO VARIFY AND LOCATE ALL DOOR OPENING AND AVOIDE PLACEMENT OF ANCHOR BOLTS IN THESE LOCATION.
9. ALL LUMBER IN CONTRACT WITH CONCRETE SHALL BE PRESSURE TREATED.
10. THE CONCRETE CONTRACTOR SHALL BACKFILL ALL RETAINING WALLS.
11. CONCRETE CONTRACTOR TO FURNISH BOLTS, NETTING AND STEEL AS REQUIRED BY THE CONCRETE DETAILS AND DRAWING.
12. SEISMIC DESIGN SHALL BE BASED ON SOIL PROFILE TYPE SD, AS RECOMMENDED.
13. FOOTINGS SHALL BE EMBEDDED ENTIRELY EITHER ON NATURAL SOIL.
14. THE BUILDING DESIGN SHALL INCORPORATE PROVISIONS FOR ANTICIPATED DIFFERENTIAL SETTLEMENTS IN EXCESS OF ONE-FOURH INCH.
15. THE SOILS ENGINEER SHALL INSPECT ALL EXCAVATIONS TO DETERMINE THAT CONDITIONS ANTICIPATED IN THE REPORT HAVE BEEN ENCOUNTERED AND TO PROVIDE AND RECOMMENDATIONS FOR THE CORRECTION OF HAZARDS FOUND DURING OR FOUNDATION EXCAVATIONS.
16. ALL MAN-MADE FILL SHALL BE COMPACTED TO A MINIMUM 90 PERCENT OF MAXIMUM DRY DENSITY OF THE FILL MATERIAL PER THE LATEST VERSION OF ASTM D 1557. WHERE COHESION LESS SOIL BE HAVING LESS THAN 15 PERCENT FINER THAN 0.005 MILLIMETERS IS USED FOR FILL, IT SHALL BE COMPACTED TO MINIMUM OF 95 PERCENT RELATIVE COMPACTION BASED ON MAXIMUM DRY DENSITY. PLACEMENT OF GRAVEL IN LIEU OF COMPACTED FILL IS ALLOWED ONLY IF COMPLYING WITH CBC 2001.
17. PRIOR TO POURING OF CONCRETE, A REPRESENTATIVE OF THE CONSULTING SOILS ENGINEER SHALL INSPECT AND APPROVE THE FOOTING EXCAVATIONS.
18. A STRUCTURE SHALL BE CONSIDERED SURCHARGING AN EXCAVATION IF THE STRUCTURE IS LOCATED WITHIN A HORIZONTAL DISTANCE FROM THE TOP OF THE EXCAVATION EQUAL TO THE DEPTH OF THE EXCAVATION
19. ALL ROOF AND PAD DRAINAGE SHALL BE CONDUCTED TO THE STREET IN AN ACCEPTABLE MANNER
20. PRIOR TO EXCAVATION, AN INITIAL INSPECTION SHALL BE CALLED AT WHICH TIME THE SEQUENCE OF SHORING (IF REQUIRED) PROTECTION FENCES AND TRAFFIC CONTROL WILL BE SCHEDULED
21. ALL FOUNDATION EXCAVATIONS MUST BE OBSERVED AND APPROVED BY THE PROJECT GEOTECHNICAL CONSULTANT PRIOR TO PLACEMENT OF REINFORCING STEEL.
22. EXCAVATIONS SHALL BE MADE IN COMPLIANCE WITH CAL/OSHA REGULATIONS.
23. NAILS IN PRESSURE TREATED WOOD SILL PLATES SHALL BE HOT DIPPED GALVANIZED, STAINLESS STEEL, SILICON BONZE OR COPPER.

Masonry

1. ALL MASONRY WORK SHALL CONFORM TO THE LATEST EDITION OF THE UNIFORM BUILDING CODE.
2. CONCRETE BLOCK SHALL BE OF SIZES SHOWN ON ARCHITECTURAL DRAWINGS AND/OR CALLED FOR IN SPECS, AND CONFORM TO A.S.T.M. C-90, GRADE "N" LIGHTWEIGHT UNITS WITH MAX. LINEAR SHRINKAGE OF 0.06
3. MORTAR MIX SHALL BE COMPOSED OF ONE PART PORTLAND CEMENT AND THREE PARTS SAND AND NOT LESS THAN 1/4" PART NOR MORE THAN 1/2 PART LINE PUTTY FOR TYPE "S" MORTAR.
4. GROUT MIX SHALL BE COMPOSED OF ONE PART PORTLAND CEMENT TO NOT MORE THAN THREE PARTS SAND AND NOT LESS THAN TWO PARTS PEA GRAVEL, MAX. GROUT LIFT = 5'-0". SPECIAL INSPECTIONS SHALL BE REQUIRED FOR HIGH LIFT GROUTING.
5. ALL MASONRY TO HAVE SPECIAL INSPECTION PER SECTION 1701.5.(7) OF UBC UNLESS NOTED OTHERWISE.
6. CEMENT FOR MORTAR AND GROUT SHALL BE LOW ALKALI TYPE CONFORMING TO ASTM C150.
7. MORTAR AND GROUT SHALL HAVE AN ULTIMATE COMPRESSIVE STRENGTH AT AGE 28 DAYS OF MORTAR f'c=1800 PSI , GROUTING f'c'= 2000 PSI & ALL MASONRY UNIT STRENGTH IS fm' = 1500 psi.

Floor Framing Notes

1. FLOOR DIAPHRAGM TO BE 19/32" PLYWOOD, PS-1-95 "CDX" WITH 10d COMMON NAILS UNBLOCKED. @6/6/10 (panel index 40/20)
2. PROVIDE 10d @6" O.C. BOUNDARY NAIL AT THE PERIMETER OF BUILDING AND AT ALL UNIT DIVISION WALLS.
3. PROVIDE 4xWIDTH OF BEAM POST OR EQUIVALENT MULTI STUD UNDER ALL BEAMS.
4. ALL FLOOR JOIST TO BE 2x 12 #2 @16 " O.C. UNLESS NOTED OTHERWISE.
5. "DBL" INDICATES 2-2x12 #2 SPIKED TOGETHER WITH 16d @9" O.C. STAGGERED WITH 3" MINIMUM EDGE DISTANCE.
6. ALL HEADERS SHALL BE 4x 8 #2 UNLESS OTHERWISE NOTED.

Reinforcing Steel

1. ALL REINFORCING SHALL BE A.S.T.M. A-615-60 FOR #5 BARS AND SMALLER. ALL REINFORCING SHALL BE A.S.T.M. A-615-60 FOR #6 BARS AND LARGER. WELDING WIRE FABRIC TO BE A.S.T.M. A-185, LAP 1-1/2 SPACES, 9"MIN. FOR STRUCTURAL SLABS, ALL REINFORCING #4 AND LARGER TO BE A.S.T.M. A-305
2. ALL BARS SHALL BE DEFORMED AS PER A.S.T.M. A-305.
3. ALL BARS SHALL BE CLEAN OF LOOSE FLAKY RUST, GREASE OR OTHER MATERIALS LIKELY TO IMPAIR BOND.
4. ALL BENDS SHALL BE MADE COLD.
5. SPLICING OF BARS SHALL HAVE LAPPING OF 40 DIA. OR 2'-0" MIN. IN ALL CONTINUOUS REINFORCEMENT OF FOOTINGS AND CONCRETE WALLS, EXCEPT AS NOTED ON PLANS. MASONRY REINFORCEMENT SHALL HAVE LAPPINGS OF 40 DIAMETER OR 2' MIN.
6. ALL REINFORCING BARS SHALL BE ACCURATELY AND SECURELY PLACED BEFORE POURING CONCRETE.
7. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE BASED ON UBC-94. COVER SHALL BE AS:
   A. POURED AGAINST EARTH............... 3"
   B. POURED AGAINST FORM BELOW GRADE......... 2"
   C. FORMED SLABS............................ 1"
   D. SLABS ON GRADE (FROM TOP TO SLAB)....... 1"
   E. WALLS EXPOSED TO WEATHER.............. 1"
   WALLS NOT EXPOSED TO WEATHER............ 1"
8. ALL FIELD WELDING OF REINFORCING STEEL REQUIRES CONTINUOUS INSPECTION BY A REGISTERED DEPUTY BUILDING INSPECTOR UNLESS NOTED OTHERWISE.
9. ALL FIELD WELDING OF REINFORCING STEEL SHALL BE PERFORMED BY WELDERS SPECIFICALLY CERTIFIED FOR REINFORCING STEEL.
10. MIN. RE-BAR SPACING KEEP 1.5d

Subterranean Specifications

1. ALL CONCRETE FOR DECK SLAB, CONCRETE BEAMS, AND CONCRETE COLUMNS TO BE f'c' = 4,000 PSI @28 DAYS WITH CONTINUOUS INSPECTION.
2. ALL CONCRETE FOR SLAB ON GRADE, MASONRY PLASTER, CONTINUOUS, ISOLATED PADS FOOTING AND PADS WITHIN CONTINUOUS FOOTING TO BE f'c' = 2,500 PSI @28 DAYS WITH CONTINUOUS INSPECTION.
3. CONCRETE BLOCK SHALL BE HOLLOW, LOAD BEARING CONCRETE MASONRY UNITS, CONFORMING TO ASTM C-90-70 GRADE N-1.
4. MORTAR OR GROUT TO BE TYPE S MORTAR WITH 1 PART CEMENT: 1/4 PART MIN. TO 1/2 PART MAX. HYDRATED LIME OR PLASTIC CEMENT: 3 PARTS SAND WITH 2,000 PSI. GROUT TO BE 1:3:2 PARTS PER GRAVEL, 2000 PSI @ 28 DAYS.
5. REINFORCING STEEL FOR CONCRETE DECK SLAB, CONCRETE COLUMNS AND ISOLATED PADS TO BE ASTM A-615 GRADE 60 (fy=60 ksi) WITH CONTINUOUS INSPECTION.
6. REINFORCING STEEL FOR MASONRY WALLS, MASONRY PLASTER, SLAB ON GRADE, CONTINUOUS FOOTING AND PADS WITHIN CONTINUOUS FOOTING TO BE ASTM A-615 GRADE 40 (fy=40 ksi) WITH NO CONTINUOUS INSPECTION.
7. MINIMUM CONCRETE COVER FOR DECK SLAB SHALL BE 1" CLEAR.
8. ALL EXTERIOR TYPE V WALLS INCLUDING THOSE @ COURT AREA TO BE ANCHORED TO DECK SLAB WITH 5/8" A.B. x 10" @32" O.C., EXCEPT WHEN NOTED ON PLANS, ALL INTERIOR TYPE V WALLS TO BE ANCHORED TO DECK SLAB WITH 1/4"Φ SHOTS @16" O.C. EXCEPT WHERE NOTED ON DECK PLAN (HILTI R.R. 2582 OR ICBO 6503).

Wood Frames

1. DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS, UNLESS OTHERWISE NOTED, AND ALSO NAILED TOGETHER WITH 16d NAILS @9" O.C..
2. UNLESS OTHERWISE NOTED ALL NAILING SHALL CONFORM TO THE NAILING SCHEDULE REQUIRED BY THE LOCAL DEPARTMENT OF THE BUILDING AND SAFETY.
3. CROSS BRIDGE FLOOR JOISTS AT 8'-0" MAXIMUM INTERVALS, MORE THAN 8" DEEP.
4. UNLESS OTHERWISE INDICATED, RAFTERS AND JOIST SHALL BE SPLICED AT SUPPORTS WITH A MINIMUM OF 4" OF LAP AND 4-16d NAILS.
5. TOP PLATE OF ALL STUD WALLS SHALL BE 2 PIECES OF 2 x 4 PLATES. SPLICES TO LAP 4'-0" MINIMUM AND BE NAILED WITH 12-16d NAILS MINIMUM EACH SIDE OF JOINT.
6. BOLT HOLES IN WOOD SHALL BE 1/32" TO 1/16" LARGER THAN THE NOMINAL BOLT DIAMETER. ALL BOLTS SHALL HAVE STANDARD CUT WASHER UNDER HEAD AND NUT UNLESS OTHERWISE NOTED.
7. PROVIDE SOLID 2 x BLOCKINGS BETWEEN JOISTS AND RAFTERS AT ALL SUPPORTS. BLOCKING SHALL BE ONE PIECE AND THE FULL DEPTH OF THE JOIST OR RAFTER.
8. USE 1 x 6 LET IN BRACE @25'-0" O.C. MINIMUM COVER 4 STUDS.
9. ALL BOLTS SHALL BE RETIGHTENED PRIOR TO THE APPLICATION OF SHEATHING, PLASTER ETC..
10. EACH SHEET OF PLYWOOD SHALL BE IDENTIFIED BY A REGISTERED STAMP OR BRAND OF THE DOGULAS FIR PLYWOOD ASSOCIATION.
11. PLYWOOD FOR ROOF SHEATHING SHALL BE CDX, USE EXTERIOR TYPE, MIN. C-C GRADE, WHERE PLYWOOD EXPOSED TO WEATHER, ALL PLYWOOD SHALL BE GLUED WITH EXTERIOR TYPE GLUE. ALL PLYWOOD SHALL CONFORM TO U.S. PRODUCT STANDARDS PS-1-74.
12. ALL WOOD BEARING ON CONCRETE OR MASONRY SHALL BE PRESSURE TREATED DOGULAS FIR.
13. STRUCTURAL MEMBERS SHALL NOT BE CUT FOR PIPES, ETC. UNLESS SPECIFICALLY DETAILED.
14. SHEATHING SHALL BE INSPECTED AND APPROVED PRIOR TO COVERING.
15. ALL SHEAR WALLS TO BE INSPECTED AND APPROVED PRIOR TO COVERING.
16. BORED HOLES SHALL NOT BE LOCATED AT THE SAME SECTION OF STUD AS A CUT OR NOTCH.
17. ALL LUMBER TO BE STRESS GRADE, DOGULAS FIR, COAST REGION AS SHOWN BELOW UNLESS OTHERWISE INDICATED ON PLANS.
    A. JOISTS AND RAFTER - #2 GRADE, 1450psi (REPETATIVE) D.F.
    B. 4 x BEAMS - #2 GRADE, 1280psi, D.F. UNLESS OTHERWISE NOTED.
    C. 4 x BEAMS WHERE NOTED ON PLANS SEL. STRUCT. 1800psi D.F.
    D. 6 x BEAMS - WHERE NOTED ON PLANS, SEL. STRUCT. 1600psi D.F.
    E. 6 x BEAMS -#1 GRADE, 1350psi, D.F..
    F. STUDS - #2 GRADE OR BETTER, D.F..
18. WOOD STUDS MAY BE NOTCHED TO A DEPTH OF 25% WIDTH OF MAXIMUM, EXCEPT INTERIOR NON-BEARING STUDS WHICH MAY BE NOTCHED TO 40% OF WIDTH MAXIMUM.
19. STUDS MAY BE BORED TO 40% OF WIDTH MAXIMUM EXCEPT INTERIOR NON-BEARING STUDS WHICH MAY BE BORED TO 60% OF WIDTH MAXIMUM WHEN EACH BORED STUD IS DOUBLED WITH NOT MORE THAN TWO SUCCESSIVE DOUBLED STUDS.
20. IN NO CASE SHALL THE EDGE OF A BORED HOLE BE NEARER THAN 5/8" TO THE EDGE OF A STUD.
21. WHERE PLUMBING, HEATING OR OTHER DUCTS ARE PLACED IN A PARTITION REQUIRING THE CUTTING OF THE DOUBLE PLATE, A STEEL ATRAP 1/8"x1-1/2"x24" SHALL BE FASTENED TO EACH SIDE OR THE TOP PLATES WITH 6-16d NAILS AT EACH END.

Concrete

1. CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C-150, TYPE II, LOW ALKALI. 1 PART CEMENT, 2 1/2 PARTS SAND, 3 1/2 PARTS GRAVEL AND 7 1/2 GALLONS WATER PER SACK OF CEMENT f'c=4000 psi EXCEPT FOUNDATION.
2. AGGREGATES SHALL BE NATURAL SAND AND ROCK CONFORMING TO ASTM C-33.
3. REINFORCING STEEL, INTERMEDIATE GRADE ASTM 1-615 GRADE 40.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM 1-185.
5. SPLICES OF HORIZONTAL REBAR IN WALLS AND FOOTHINGS SHALL BE STAGGERED.
6. DOWELS FOR WALL SHALL BE SAME SIZE AND SPACING AS THE WALL REINFORCEMENT AND SHALL LAP WITH THE WALL REBAR AS NOTED ABOVE UNLESS NOTED OTHERWISE.
7. ANCHOR BOLTS, DOWELS, INSERTS ETC. SHALL BE SECURELY TIED IN PLACE PRIOR TO THE POURING OF ANY CONCRETE OF GROUT.
8. WELDING OF REINFORCING STEEL SHALL CONFORM TO AWS D12-1 USING PROPER LOW HYDROGEN ELECTRODES.
9. ALL EXCAVATIONS SHALL BE FREE OF LOOSE DIRT, WATER OR DEBRIS PRIOR TO POURING CONCRETE.
10. ALL FORMS SHALL BE CONSTRUCTED AS TO MAINTAIN THE REQUIRED POSITION AND SHAPE DURING AND AFTER THE PLACING OF CONCRETE AND BE SUFFICIANTLY TIGHT TO PREVENT THE LEAKAGE OF CONCRETE.
11. NO PIPES OR DUCTS ARE TO BE PLACED IN CONCRETE SLABS OR WALLS UNLESS SPECIFICIALLY DETAILED.
12. ALL CONCRETE SHALL CONFORM TO THE LATEST EDITION OF THE UNIFORM BUILDING CODE.
13. ALL CONCRETE WHICH IS Fc'≥2500 psi SHALL HAVE SPECIAL INSPECTION PER SECTION 306 OF THE UNIFORM BUILDING CODE UNLESS NOTED OTHERWISE.
14. PNEUMATICALLY PLACED CONCRETE SHALL HAVE A-28-DAY STRENGTH OF 3000 psi MATERIALS, PROPORTIONING AND APLICATION, SHALL CONFORM WITH ACI STANDARD 506.2-77. THE CONCRETE SHALL BE PLACED BY A QUALIFIED CONTRACTOR AND UNDER CONTINUOUS INSPECTION BY A REGISTERED DEPUTY BUILDING INSPECTOR.
15. CONCRETE STRENGTH IS f'c = 3000 psi (28 DAYS) FOR CONC. SLAB ON GRADE, & f'c=3000psi(28DAYS) FOR CONC. FOOTING.

Roof Framing Notes

1. ROOF DIAPHRAGM TO BE 15/32" PLYWOOD, PS-1-95 "CDX" WITH 10d COMMON NAILS UNBLOCKED. @6/6/12 (panel index 32/16)
2. PROVIDE 10d @6" O.C. BOUNDARY NAIL AT THE PERIMETER OF BUILDING AND AT ALL UNIT DIVISION WALLS.
3. PROVIDE 4xWIDTH OF BEAM POST OR EQUIVALENT MULTI STUD UNDER ALL BEAMS.
4. ALL ROOF RAFTERS TO BE 2x 10 #2 @ 24 " O.C. UNLESS NOTED OTHERWISE.
5. ALL HEADERS SHALL BE 4x 6 #2 UNLESS OTHERWISE NOTED.

Structural Steel

1. STRUCTURAL STEEL SHALL CONFORM TO ASTM A-992 EXCEPT FOR PIPE COLUMNS AND SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC SPECIFICATION FOR DESIGN, FABRICATION AND CONSRUCTION OF STRUCTURAL STEEL FOR BUILDINGS.
2. ALL SHOP FABRICATION SHALL BE PERFORMED BY A FABRICATOR LICENSED BY THE CITY BUILDING DEPARTMENT.
3. WELDING SHOULD CONFORM TO CITY OF SANTA MONICA AND THE LATEST AMERICAN WELDING SOCIETY AWS STANDARDS. WELDING SHALL BE PERFORMED BY A PROCESS APPROVED BY THE ENGINEER AND THE BUILDING DEPARTMENT.
4. ALL FIELD WELDING SHALL BE REQUIRED CONTINUOUS INSPECTION
5. ALL STEEL TO STEEL TO WOOD BOLTED CONNECTION SHOULD BE MADE WITH COMMON BOLTS 307, LOCK NUTS AND LOCK WASHERS SHALL BE PROVIDED UNDER THE NUTS.
6. FILLS SHALL BE PROVIDED AT SPLICES OF PARTS HAVING MORE THAN 1/8 INCH DIFFERENCE IN THICKNESS.
7. PIPE AND TUBE MEMBERS SHALL CONFIRM TO ASTM A-501 OR A-53 AS REQUIRED.
8. ALL PIPE SHALL BE ASTM A-53, GRADE "B".
9. ALL TUBULAR STEEL SECTIONS SHALL CONFIRM TO A.S.T.M. A-36.
10. NO STRUCTURAL STEEL SHALL BE FABRICATED OR ERECTED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED BY THE STRUCTURAL ENGINEER.
11. NO FIELD CUTTING OR BURING OF STRUCTURAL STEEL WILL BE PERMITTED WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
12. BASE PLATE SHALL BE LEVELED WITH DOUBLE NUTS, NO LEVELING PLATE ALLOWED.
13. BOLT HOLE IN STEEL SHALL BE 1/16" LARGER THAN NOMINAL BOLT SIZE.
14. BOLTS SHALL CONFORM TO A.S.T.M. A-307 UNLESS OTHERWISE NOTED ON PLANS.
15. GAGE METAL SHALL CONFORM TO A.S.T.M. A-245.
16. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL NECESSARY BRACING.
17. ALL SUBSTITUTIONS SUBJECT TO BUILDING DEPARTMENT APPROVAL.
18. CONTINUOUS INSPECTION IS REQUIRED FOR FIELD WELDING AND SHOP WELDING TO BE DONE BY AN APPROVED CERTIFIED WELDER.
19. THE SEISMIC DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH PART I (LRFD) AND PART III (ASD) OF THE SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, APRIL 15, 1997, PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC). THESE PROVISIONS SHALL BE APPLIED IN CONJUNCTION WITH CHAPTER 22, DIVISION II.
20. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE ENGINEER OF RECORD. SPECIFY THE REQUIRED "WELDING PROCEDURE SPECIFICATION" ON PLANS.
21. ALL COMPLETE-JOINT-PENETRATION GROOVE WELDS USED IN THE SEISMIC FORCE RESISTING SYSTEM SHALL BE MADE WITH A FILLER METAL THAT HAS A MINIMUM CVN TOUGHNESS OF 20 FT-LB AT MINUS 20° F.
22. DISCONTINUITIES IN WELD CREATED BY ERRORS OR BY FABRICATION OR ERECTION OPERATIONS, SUCH AS TRACK WELDS, ERECTION AIDS, AIR-ARC GOUGING AND FLAME CUTTING, SHALL BE REPAIRED AS REQUIRED BY THE ENGINEER OF RECORD.
23. FOR CONNECTIONS WITH WELDED FLANGE JOINTS, WELD BACKING AND RUN-OFF TABS SHALL BE REMOVED AND REPAIRED INCLUDING THE USE OF A REINFORCING FILLET WELD, EXCEPT THAT THE TOP-FLANGE BACKING IS PERMITTED TO REMAIN IN PLACE IF IT IS ATTACHED TO THE COLUMN FLANGE WITH A CONTINUOS FILLET WELD ON THE EDGE BELOW THE COMPLETE-JOINT-PENETRATION GROOVE WELD.
24. THE USE OF ROLLED STEEL SECTIONS, BOLTS AND OR REBAR MANUFACTURED OUTSIDE THE UNITED STATES WILL REQUIRE VERIFICATION THAT THE PRODUCTS COMPLY WITH APPLICABLE ASTM STANDARDS. MILL CERTIFICATES WILL BE REQUIRED FOR ALL STEEL. STEEL GRADES OTHER THAN ASTM - 36 WILL REQUIRE TESTING BY AN APPROVED LABORATORY. ALL FOREIGN BOLTS MUST BE APPROVED BY THE CITY BUILDING AND SAFETY DEPARTMENT PRIOR TO THEIR USE.

Special Inspection

1. CONCRETE
   f'c DESIGN STRENGTH GREATER THAN 2,500psi.
   PLACING OF REINFORCING STEEL.
2. WELDING
   DUCTILE MOMENT RESISTING STEEL FRAMES.
   STRUCTURAL STEEL.
   REINFORCING STEEL.
   SHEAR CONNECTORS.
   ANCHOR STUDS.
3. HIGH STRENGTH BOLTING
4. GRADE BEAM
5. BOLTS INSTALLED IN CONCRETE
6. EPOXY GROUTED ANCHOR BOLTS AND REINFORCING

STRUCTURAL NOTES
(NAILING SCHEDULE)

OBSERVED STRUCTURAL ELEMENT AND THEIR CONNECTIONS

	CONNECTION	NAILING[1]
1.	JOIST TO SILL OR GIRDER, TOENAIL	3-8d
2.	BRIDGING TO JOIST, TOENAIL EACH END	2-8d
3.	1"x6" (25mmx152mm) SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
4.	WIDER THAN 1"x6" (25mmx152mm) SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
5.	2" (51mm) SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
6.	SOLE PLATE JOIST OR BLOCKING, TYPICAL FACE NAIL SOLE PLATE JOIST OR BLOCKING, AT BRACED WALL PANELS	16d AT 16"(406mm)O.C. 3-16d PER 16" (406MM)
7.	TOP PLATE TO STUD, END NAIL	2-16d
8.	STUD TO SOLE PLATE	4-8d. TOENAIL OR 2-16d. END NAIL
9.	DOUBLE STUDS, FACE NAIL	16d AT 24"(610mm) O.C.
10.	DOUBLED TOP PLATES, TYPICAL FACE NAIL DOUBLED TOP PLATES, LAP SPLICE	16d AT 16"(406mm) O.C. 8-16d
11.	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	3-8d
12.	RIM JOIST TO TOP PLATE, TOENAIL	8d AT 6"(152mm) O.C.
13.	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
14.	CONTINUOUS HEADER, TWO PIECES	16d AT 16"(406mm)O.C. ALONG EACH EDGE
15.	CEILING JOISTS TO PLATE, TOENAIL	3-8d
16.	CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
17.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
18.	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
19.	RAFTER TO PLATE, TOENAIL	3-8d
20	1" (25mm) BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
21.	1"x8" (25mmx203mm) SHEATHING OR LESS TO EACH BEARING, FACE NAIL	2-8d
22	WIDER THAN 1"x8" (25mmx203mm) SHEATHING TO EACH BEARING, FACE NAIL	3-8d
23	BUILT-UP CORNER STUDS	16d AT 24"(610mm) O.C.
24	BUILT-UP GIRDER AND BEAMS	20d AT 32"(813mm) O.C. AT TOP AND BOTTOM AND STAGGERED 2-20d AT ENDS AND AT EACH SPLICE
25	2" (51mm) PLANKS	2-16d EACH BEARING
26	WOOD STRUCTURAL PANELS AND PARTICLEBOARD :[2]	
	SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING):(1 INCH=25.4mm)	
	1/2" (12.7mm) AND LESS	6d[3]
	19/32" -3/4" (15mm-19mm)	8d[4] OR 6d[5]
	7/8" - 1" (22mm-25mm)	8d[3]
	1 1/8" - 1 1/4" (29mm-32mm)	10d[4] OR 8d[5]
	COMBINATION SUBFLOOR-UNDERLAYMENT (TO FRAMING):(1 INCH=25.4mm)	
	3/4" (19mm) AND LESS	6d[5]
	7/8" - 1" (22mm-25mm)	8d[5]
	1 1/8" - 1 1/4" (29mm-32mm)	10d[4] OR 8d[5]
27	PANEL SIDING (TO FRAMING)[2]:	
	1/2" (12.7mm) OR LESS	6d[6]
	5/8" (16mm)	8d[6]
28	FIBERBOARD SHEATHING :[7]	
	1/2" (12.7mm)	NO. 11 ga.[8] 6d[4] NO. 16 ga.[9]
	25/32" (20mm)	NO. 11 ga.[8] 8d[4] NO. 16 ga.[9]
29	INTERIOR PANELING	
	1/4" (6.4mm)	4d[10]
	3/8" (9.5mm)	6d[11]

1. COMMON OR BOX NAILS MAY BE USED EXCEPT WHERE OTHERWISE STATED.
2. NAILS SPACED AT 6" O.C. AT EDGES, 12" AT INTERMEDIATE SUPPORTS EXCEPT 6" SPANS ARE 48" OR MORE. FOR NAILING OF WOODAT ALL SUPPORTS WHERE STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTIONS 2315.3.3 AND 2315.4. NAILS FOR WALL SHEATHING MAY BE COMMON, BOX OR CASING.
3. COMMON OR DEFORMED SHANK.
4. COMMON.
5. DEFORMED SHANK.
6. CORROSION-RESISTANT SIDING OR CASING NAILS CONFORMING TO THE REQUIREMENTS OF SECTION 2304.3.
7. FASTENERS SPACED 3" O.C. AT EXTERIOR EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS.
8. CORROSION-RESISTANT ROOFING NAILS WITH 7/16"-DIAMETER HEAD AND 1 1/2" LENGTH FOR 1/2" SHEATHING AND 1 3/4" LENGTH FOR 25/32" SHEATHING CONFORMING TO THE REQUIREMENTS OF SECTION 2304.3.
9. CORROSION-RESISTANT STAPLES WITH NORMINAL 7/16" CROWN AND 1 1/8" LENGTH FOR 1/2" SHEATHING AND 1 1/2" LENGTH FOR 25/32" SHEATHING CONFORMING TO THE REQUIREMENTS OF SECTION 2304.3.
10. PANEL SUPPORTS AT 16", 20" IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, U.N.O.
    CASING OR FINISH NAILS SPACED 6" ON PANEL EDGES, 12" AT INTERMEDIATE SUPPORTS.
11. PANEL SUPPORTS AT 24". CASING OR FINISH NAILS SPACED 6" ON PANEL EDGES, 12" AT INTERMEDIATE SUPPORTS.

CITY OF SANTA MONICA
DEPARTMENT OF BUILDING & SAFETY
GENERAL NOTES FOR STRUCTURAL OBSERVATION

1. Structural observation is required for the structural system in accordance with MGD 110. Structural observation is the visual observation of the elements and connections of the structural system at significant construction stages and the completed structure for general conformance to the approved plans and specifications. Structural observation does not waive the responsiblility for the inspections required of the building inspector or the deputy inspector.

2. The owner shall employ a civil or structural engineer or architect to perform the structural observation. The engineer or architect shall registered or licensed in the State of California. The department of Building and Safety recommends the use of the engineer or architect responsible for the structural design when they are independent of the contractor.

3. The structural observer shall provide evidence of employment by the owner. A letter from the owner or a copy of the agreement for services shall be sent to the building inspector before the first site visit. The structural observer shall also inform the owner of the requirements for a pre construction meeting and shall preside over this meeting.

4. The owner or owner's representative shall coordinate and call for a meeting between the engineer or architect responsible for the structural design, structural observer, contractor, affected subcontractors and deputy inspectors. The purpose of the meeting shall be to identify the major structural elements and connections that affect the vertical and lateral load systems of the structure and to review scheduling of the required observations. A record of the meeting shall be included in the first observation report submitted to the building inspector.

5. The structural observer shall perform site visits at those steps in the progress of the work that allow for correction of deficiencies without substantial effort or uncovering of the work involved. At a minimum, the following significant construction stages require a site visit and an observation report from the structural observer :

	Construction Stages		Elements/Connections to be Observed
a)	FOUNDATION	/	SIZE & RE-BAR
b)	CONCRETE	/	RE-BAR
c)	WOOD	/	SHEAR WALL / DIAPHRAGM
d)	MASONRY	/	RE-BAR & DOWEL
e)	STEEL	/	CONNECTION
f)	______	/	______

6. The structural observer shall prepare a report on the Department form B&S 261 for each significant stage of construction observed. The original of the observation report shall be sent to the building inspector's office and shall be signed and sealed (web stamp) by the responsible structural observer. One copy of the observation report shall be attached to the approved plans. The copy attached to the plans need not be sealed but shall be signed by the responsible structural observer or their designee. Copies of the report shall also be given to the owner, contractor, and deputy inspector.

7. A final observation report must be submitted which shows that all observed deficiencies were resolved and the structural system generally conforms with the approved plans and specifications. The Department of Building and Safety will not accept the structural work without this final observation report and the correction of specific deficiencies noted during normal building and deputy inspection.

8. The structural observer shall send the original observation report to be following inspection office :
   Inspection Group Name ______
   Street Address ______
   Community of SANTA MONICA, CA, Zip Code ______

9. When the owner elects to change the structural observer of record, the owner shall :
   a) notify the building inspector in writing before the next inspection,
   b) call an additional pre construction meeting, and.
   c) furnish the replacement structural observer with a copy of all previous observation reports.

   The replacement structural observer shall approve the correction of the original observed deficiencies unless otherwise approved by plan check supervision. The policy of the Department shall be to correct any properly noted deficiencies without consideration of their source.

10. The engineer or architect of record shall develop all changes relating to the structural systems. The building department shall review and approve all changes to the approved plans and specifications.

No.	REVISION	DATE

★ REGISTERED PROFESSIONAL ENGINEER ★ SANG YOUCK LEE No. 3821 Exp.03-31-08 STRUCTURAL STATE OF CALIFORNIA

MR. ALI MADANI & MR.HOSSEIN ASHTIANI
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